

STRUCTURAL CALCULATION

Deck beams HxB 40x60

CLIENT: Felix Clercx Handelsonderneming

PROJECT NUMBER: 18.124-3

DATE: 12-2-2019

REVISION: -

ENGINEER : ing. E. Rooijackers

CONTENTS

GENERAL INFORMATION 2

- * DESCRIPTION..... 2
- * APPLICABLE CODES REGULATIONS 2
- * MATERIAL..... 2
- * BUILDING DATA 2
- * SAFETY FACTORS 2
- * LOADS 3

BEAMS..... 4

- *Category A*..... 5
- *Category B, C and D*..... 12

GENERAL INFORMATION

* Description

This report calculates the aluminium duct profiles that are applied in a deck system. The beams are centred at 400mm.

- max. span = 100 cm in category A buildings (houses and apartments)
- max. span = 45 cm in category B, C and D buildings

* Applicable codes regulations

EN 1990 Eurocode 0: Basis of structural design

EN 1991 Eurocode 1: Actions on structures

EN 1999 Eurocode 9: Design and calculation of aluminium structures

* Material

Aluminium : - EN AW 6060 T5 (permissible stress $\leq 150 \text{ N/mm}^2$)

* Building data

- Building type :
Category A: Living accommodation
Category B: Offices
Category C: Meeting function, not category A, B, or D
Category D: Shops
- Effect Classes:
CC1 (cat. A)
CC2 (cat. B, C and D)
- Reliability class:
RC1, $k_{fi} = 0,90$ (cat. A)
RC2 $k_{fi} = 1.00$ (cat. B, C and D)
- Design Lifetime class:
3 (50 years)

* Safety factors

Category A

Permanent: $\gamma_g = 1.22 / 1.08 / 0.9$

Imposed : $\gamma_q = 1.35$

Time factor : $\phi_t = 1.0$

Category B, C and D

Permanent: $\gamma_g = 1.35 / 1.2 / 0.9$

Imposed : $\gamma_q = 1.5$

Time factor : $\varphi_t = 1.0$

* Loads

- Wood or composite deck parts

$$g_k = 0.30 \text{ kN/m}^2$$

- Imposed loads category A

$$q_k = 2.50 \text{ kN/m}^2$$

$$Q_k = 3.00 \text{ kN on a surface of } 0.5 \times 0.5 \text{ m}^2, \quad \varphi_0 = 0.4, \varphi_1 = 0.5, \varphi_2 = 0.3$$

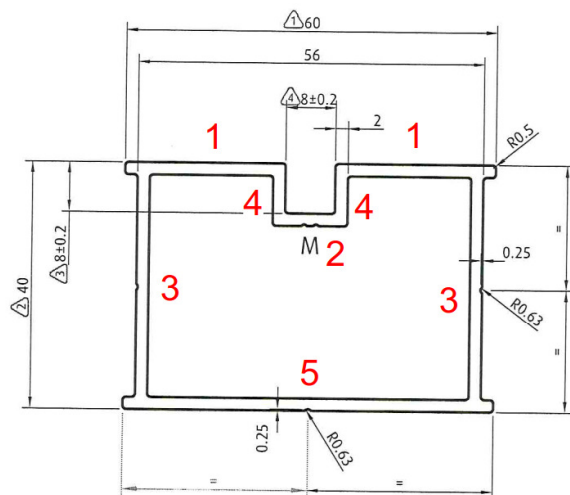
- Imposed loads category B, C and D

$$q_k = 5.00 \text{ kN/m}^2$$

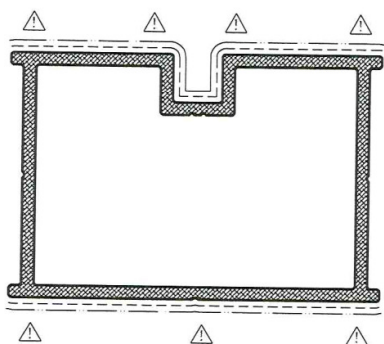
$$Q_k = 7.00 \text{ kN on a surface of } 0.5 \times 0.5 \text{ m}^2, \quad \varphi_0 = 0.5, \varphi_1 = 0.5, \varphi_2 = 0.3$$

BEAMS

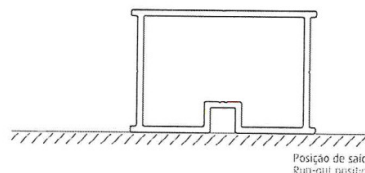
Cotas não tolerânciadas de acordo com NP EN 12020-2
Unspecified tolerances according NP EN 12020-2



Esc. 1:1



Esc. 1:1



Posição de saída
Run-out position

Rev. Nº					Des:	Data:	Aprov:
Rev. Nº					Des:	Data:	Aprov:
Liga Alloy	EN AW 6060	Esp. Geral Unspec. Thickness	2.0 mm	A. Anod. Anod. Area	0,221 m2/m	Ref. Prov. Prov. Ref.	X.07.11.17 - 1
Trat. Térmico Heat Treatment	T5	Per. Int. Int. Per.	191 mm	A. Polir. Polishing Area	0,135 m2/m	Desen. Drawn	Ana Rosa Date 07.11.17
Diam. Circunscrito Circumscribed Diam.	Ø 72 mm	Per. Ext. Ext. Per.	221 mm	lx:	cm4	Verif. Verif.	JPS Date 13.11.2017
Peso Weight	1,121 kg/m	Per. Total Total Per.	412 mm	Raios n colados	mm	Aprov.	Data
						Referência Reference	NZ.664.007
						Ref. Cliente Client Ref.	
						Cliente	

- Category A

Compiled profile

	Width [mm]	Height [mm]	e [mm]	I _y part [mm ⁴]	I shifted [mm ⁴]	I tot [mm ⁴]
part 1	52	2	39	35	34.566	34.600
part 2	12	2	31	8	2.512	2.520
part 3	4	36	20	15.552	85	15.637
part 4	4	6	35	72	4.860	4.932
part 5	60	2	1	40	46.899	46.939

Total Height: 40 mm

Centre of gravity : 20,8 mm

Surface: 416 mm²

Weight: 3,3 kg/m

I tot : 104.629 mm⁴

Wel : 5.038 mm³

In terms of static values, the profile corresponds to a **RHS 60x40x2**

The beams are centred at 400mm. The support distance may not exceed the maximum of 1000mm

$$g_k = 0.4 \times 0.30 = 0.12 \text{ kN/m}$$

Category A

$$q_k = 0.4 \times 2.50 = 1.00 \text{ kN/m}$$

$$Q_k = 3.00 \text{ kN}$$

Due to a limitation in the computer calculation program, the strength of the profile is introduced as steel instead of aluminium. It will be taken into account that the stress does not exceed 150 N/mm². For the E-modulus, used for calculating the deflection, the value of aluminum is used.

Technosoft Liggers release 6.29

12 feb 2019

Project.....: 18124-1 -
Onderdeel....: liggers 40x24x2, categorie A
Constructeur.: Emiel Rooijackers
Opdrachtgever:
Dimensies....: kN/m/rad
Datum.....: 22/11/2018
Bestand.....: d:\dropbox\projecten\2018\18124\berekeningen\liggers
60x40x2, categorie a.dlw

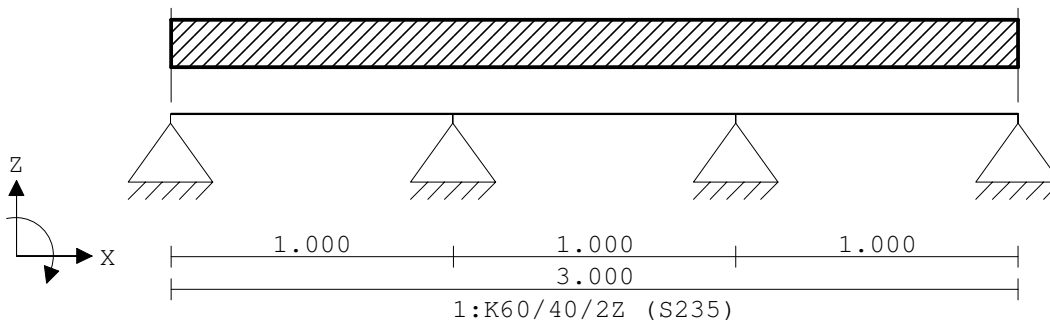
Betrouwbaarheidsklasse : 1 Referentieperiode : 50

Toegepaste normen volgens Eurocode met Nederlandse NB

Belastingen	NEN-EN 1990:2002	C2:2010	NB:2011(nl)
	NEN-EN 1991-1-1:2002	C1:2009	NB:2011(nl)
Staal	NEN-EN 1993-1-1:2006	C2:2011,A1:2016	NB:2016(nl)

GEOMETRIE

Ligger:1



PROFIELVORMEN [mm]

1 K60/40/2Z



BELASTINGGEVALLEN

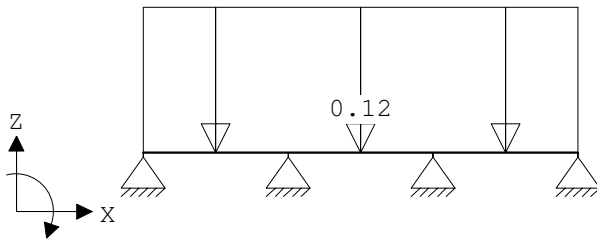
B.G.	Omschrijving	Belast/onbelast	Ψ_0	Ψ_1	Ψ_2	e.g.
1	Permanent	2:Permanent EN1991				-1.00
2	Veranderlijk lijnlas	1:Schaakbord EN1991	0.40	0.50	0.30	0.00
3	Veranderlijk puntlas	3:Kraanbaan	0.40	0.50	0.30	0.00

BELASTINGGEVALLEN

B.G.	Omschrijving	Type
1	Permanent	1 Permanente belasting
2	Veranderlijk lijnlast	2 Ver. bel. pers. ed. (p_rep)
3	Veranderlijk puntlast	3 Ver. bel. pers. ed. (F_rep)

VELDBELASTINGEN

Ligger:1 B.G:1 Permanent



VELDBELASTINGEN

Ligger:1 B.G:1 Permanent

Last Ref.	Type	Omschrijving	q1/p/m	q2	psi	Afstand	Lengte
1	1:q-last		-0.120	-0.120		0.000	3.000

REACTIES

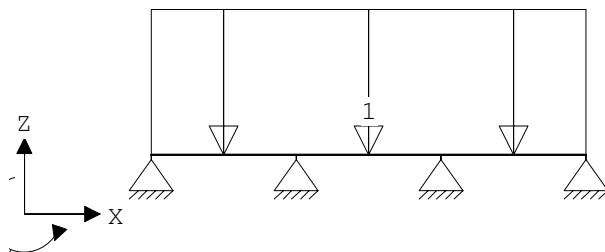
Ligger:1 B.G:1 Permanent

Stp	F	M
1	0.05	0.00
2	0.14	0.00
3	0.14	0.00
4	0.05	0.00

0.39 : (absoluut) grootste som reacties
-0.39 : (absoluut) grootste som belastingen

VELDBELASTINGEN

Ligger:1 B.G:2 Veranderlijk lijnlast



VELDBELASTINGEN

Ligger:1 B.G:2 Veranderlijk lijnlast

Last Ref.	Type	Omschrijving	q1/p/m	q2	psi	Afstand	Lengte
1	1:q-last		-1.000	-1.000		0.000	3.000

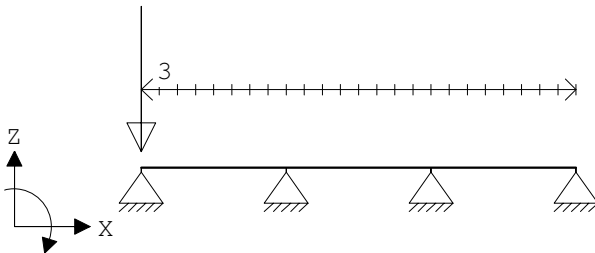
REACTIES

Ligger:1 B.G:2 Veranderlijk lijnlast

Stp	Fmin	Fmax	Mmin	Mmax
1	-0.05	0.45	0.00	0.00
2	0.00	1.20	0.00	0.00
3	0.00	1.20	0.00	0.00
4	-0.05	0.45	0.00	0.00

VELDBELASTINGEN

Ligger:1 B.G:3 Veranderlijk puntlast



VELDBELASTINGEN

Ligger:1 B.G:3 Veranderlijk puntlast

Last Ref.	Type	Omschrijving	q1/p/m	q2	psi	Afstand	Lengte
1	15:Pass.stelsel		-3.000	0.125		0.000	3.000

REACTIES

Ligger:1 B.G:3 Veranderlijk puntlast

Stp	Fmin	Fmax	Mmin	Mmax
1	-0.24	3.00	0.00	0.00
2	-0.46	3.00	0.00	0.00
3	-0.46	3.00	0.00	0.00
4	-0.24	3.00	0.00	0.00

BELASTINGCOMBINATIES

BC	Type				
1	Fund.	1.22	$G_{k,1}$		
2	Fund.	0.90	$G_{k,1}$		
3	Fund.	1.22	$G_{k,1}$	+	1.35 $\Psi_0 Q_{k,2}$
4	Fund.	1.22	$G_{k,1}$	+	1.35 $\Psi_0 Q_{k,3}$
5	Fund.	1.08	$G_{k,1}$	+	1.35 $Q_{k,2}$
6	Fund.	1.08	$G_{k,1}$	+	1.35 $Q_{k,3}$
7	Fund.	0.90	$G_{k,1}$	+	1.35 $\Psi_0 Q_{k,2}$
8	Fund.	0.90	$G_{k,1}$	+	1.35 $Q_{k,2}$
9	Fund.	0.90	$G_{k,1}$	+	1.35 $Q_{k,3}$
10	Fund.	0.90	$G_{k,1}$	+	1.35 $\Psi_0 Q_{k,3}$
11	Kar.	1.00	$G_{k,1}$	+	1.00 $Q_{k,2}$
12	Kar.	1.00	$G_{k,1}$	+	1.00 $Q_{k,3}$
13	Blij.	1.00	$G_{k,1}$		

GUNSTIGE WERKING PERMANENTE BELASTINGEN

BC	Velden met gunstige werking
1	Geen
2	Alle velden de factor:0.90
3	Geen
4	Geen
5	Geen
6	Geen
7	Alle velden de factor:0.90

GUNSTIGE WERKING PERMANENTE BELASTINGEN

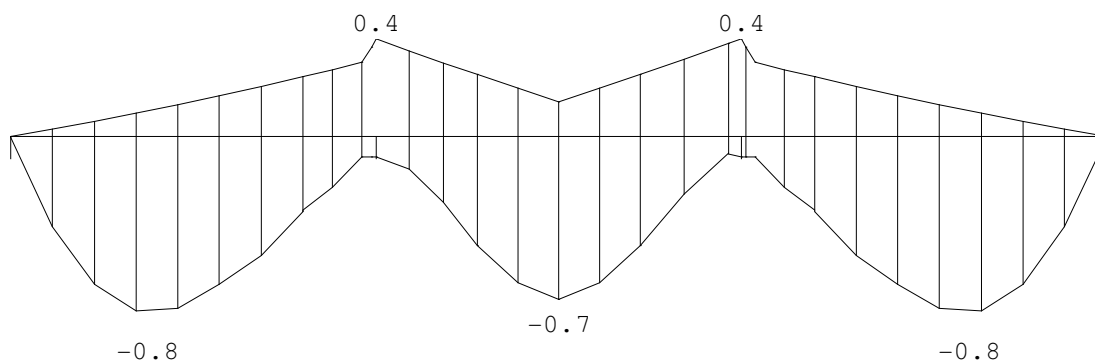
BC Velden met gunstige werking

- 8 Alle velden de factor:0.90
- 9 Alle velden de factor:0.90
- 10 Alle velden de factor:0.90

OMHULLENDE VAN DE FUNDAMENTELE COMBINATIES

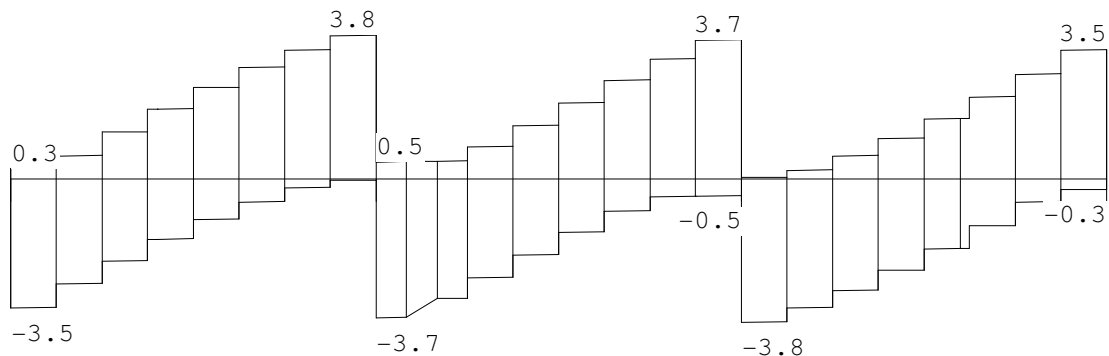
MOMENTEN

Ligger:1 Fundamentele combinatie



DWARSKRACHTEN

Ligger:1 Fundamentele combinatie



Fmin:-0.28 -0.49 -0.49 -0.28
Fmax:4.11 4.20 4.20 4.11

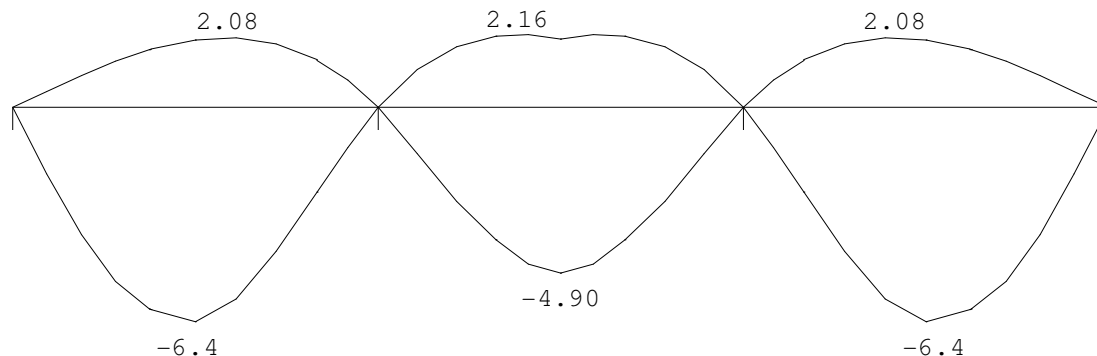
REACTIES

Ligger:1 Fundamentele combinatie

Stp	Fmin	Fmax	Mmin	Mmax
1	-0.28	4.11	0.00	0.00
2	-0.49	4.20	0.00	0.00
3	-0.49	4.20	0.00	0.00
4	-0.28	4.11	0.00	0.00

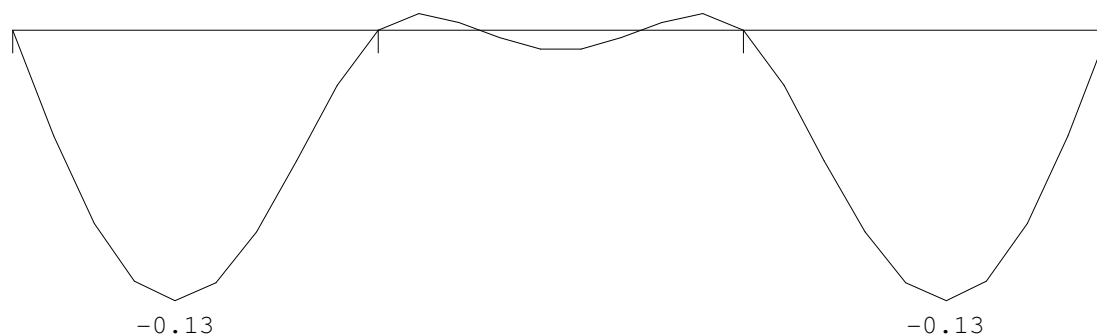
OMHULLENDE VAN DE KARAKTERISTIEKE COMBINATIES

VERPLAATSINGEN [mm] Ligger:1 Karakteristieke combinatie



OMHULLENDE VAN DE BLIJVENDE COMBINATIES

VERPLAATSINGEN [mm] Ligger:1 Blijvende combinatie



STAALPROFIELEN - ALGEMENE GEGEVENS

Ligger:1

Stabiliteit: Classificatie gehele constructie: Geschoord

MATERIAAL

Mat nr.	Profielnaam	Vloeis. [N/mm ²]	Productie methode	Min. drsn. klasse
1	K60/40/2Z	235	Warmgewalst	1

Partiële veiligheidsfactoren:

Gamma M;0 : 1.00 Gamma M;1 : 1.00

KIPSTABILITEIT

Ligger:1

Staaft.	Plts. aangr.	l gaffel	Kipsteunafstanden
		[m]	[m]
1	1.0*h	boven:	1.00 1.000
		onder:	1.00 1.000
2	1.0*h	boven:	1.00 1.000
		onder:	1.00 1.000
3	1.0*h	boven:	1.00 1.000
		onder:	1.00 1.000

TOETSING SPANNINGEN

Ligger:1

Staaft nr.	Mat	BC	Sit	Kl	Plaats	Norm	Artikel	Formule	Hoogste toetsing U.C. [N/mm ²]	Opm.
1	1	6	4	1	Mz-max	EN3-1-1	6.2.8	(6.29)	0.612 144	< 150
2	1	6	13	1	Mz-max	EN3-1-1	6.2.8	(6.29)	0.526 124	
3	1	6	22	1	Mz-max	EN3-1-1	6.2.8	(6.29)	0.612 144	

- Category B, C and D

The beams are centred at 400mm. The support distance may not exceed the maximum of 450mm

$$g_k = 0.4 \times 0.30 = 0.12 \text{ kN/m}$$

Category B, C and D

$$q_k = 0.4 \times 5.00 = 2.00 \text{ kN/m}$$

$$Q_k = 7.00 \text{ kN}$$

Due to a limitation in the computer calculation program, the strength of the profile is introduced as steel instead of aluminium. It will be taken into account that the stress does not exceed 150 N/mm^2 . For the E-modulus, used for calculating the deflection, the value of aluminum is used.

Technosoft Liggers release 6.29

12 feb 2019

Project.....: 18124-1 -
Onderdeel....: liggers 40x24x2, categorie A
Constructeur.: Emiel Rooijackers
Opdrachtgever:
Dimensies....: kN/m/rad
Datum.....: 22/11/2018
Bestand.....: d:\dropbox\projecten\2018\18124\berekeningen\liggers
60x40x2, categorie b, c en d.dlw

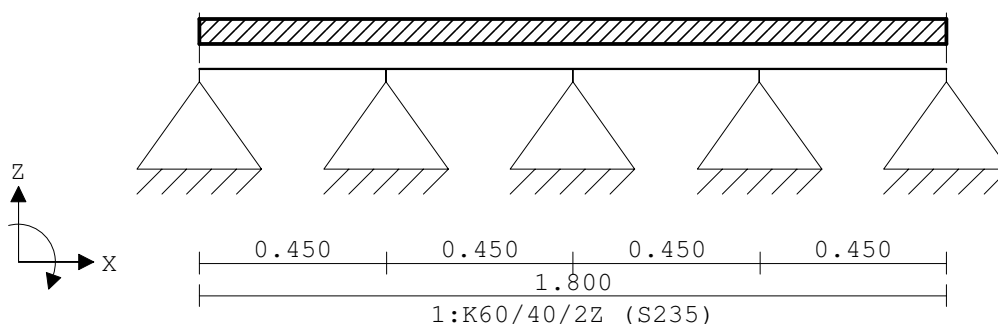
Betrouwbaarheidsklasse : 1 Referentieperiode : 50

Toegepaste normen volgens Eurocode met Nederlandse NB

Belastingen	NEN-EN 1990:2002	C2:2010	NB:2011(nl)
	NEN-EN 1991-1-1:2002	C1:2009	NB:2011(nl)
Staal	NEN-EN 1993-1-1:2006	C2:2011,A1:2016	NB:2016(nl)

GEOMETRIE

Ligger:1



PROFIELVORMEN [mm]

1 K60/40/2Z



BELASTINGGEVALLEN

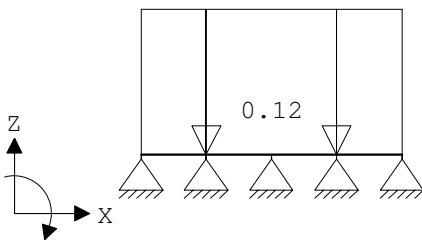
B.G.	Omschrijving	Belast/onbelast	Ψ_0	Ψ_1	Ψ_2	e.g.
1	Permanent	2:Permanent EN1991				-1.00
2	Veranderlijk lijnlas	1:Schaakbord EN1991	0.50	0.50	0.30	0.00
3	Veranderlijk puntlas	3:Kraanbaan	0.40	0.50	0.30	0.00

BELASTINGGEVALLEN

B.G.	Omschrijving	Type
1	Permanent	1 Permanente belasting
2	Veranderlijk lijnlast	2 Ver. bel. pers. ed. (p_rep)
3	Veranderlijk puntlast	3 Ver. bel. pers. ed. (F_rep)

VELDBELASTINGEN

Ligger:1 B.G:1 Permanent



VELDBELASTINGEN

Ligger:1 B.G:1 Permanent

Last Ref.	Type	Omschrijving	q1/p/m	q2	psi	Afstand	Lengte
1	1:q-last		-0.120	-0.120		0.000	1.800

REACTIES

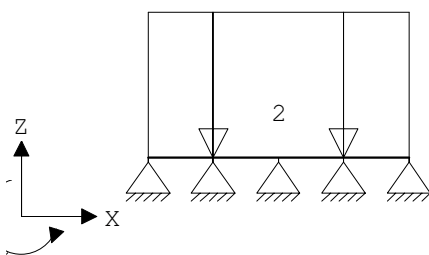
Ligger:1 B.G:1 Permanent

Stp	F	M
1	0.02	0.00
2	0.07	0.00
3	0.05	0.00
4	0.07	0.00
5	0.02	0.00

0.23 : (absoluut) grootste som reacties
-0.23 : (absoluut) grootste som belastingen

VELDBELASTINGEN

Ligger:1 B.G:2 Veranderlijk lijnlast



VELDBELASTINGEN

Ligger:1 B.G:2 Veranderlijk lijnlast

Last Ref.	Type	Omschrijving	q1/p/m	q2	psi	Afstand	Lengte
1	1:q-last		-2.000	-2.000		0.000	1.800

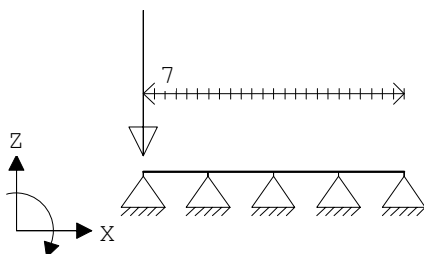
REACTIES

Ligger:1 B.G:2 Veranderlijk lijnlast

Stp	Fmin	Fmax	Mmin	Mmax
1	-0.05	0.40	0.00	0.00
2	0.00	1.10	0.00	0.00
3	0.00	1.03	0.00	0.00
4	0.00	1.10	0.00	0.00
5	-0.05	0.40	0.00	0.00

VELDBELASTINGEN

Ligger:1 B.G:3 Veranderlijk puntlast


VELDBELASTINGEN

Ligger:1 B.G:3 Veranderlijk puntlast

Last Ref.	Type	Omschrijving	q1/p/m	q2	psi	Afstand	Lengte
1	15:Pass.stelsel		-7.000	0.075		0.000	1.800

REACTIES

Ligger:1 B.G:3 Veranderlijk puntlast

Stp	Fmin	Fmax	Mmin	Mmax
1	-0.55	7.00	0.00	0.00
2	-0.89	7.00	0.00	0.00
3	-1.13	7.00	0.00	0.00
4	-0.89	7.00	0.00	0.00
5	-0.55	7.00	0.00	0.00

BELASTINGCOMBINATIES

BC	Type			
1	Fund.	1.22	$G_{k,1}$	
2	Fund.	0.90	$G_{k,1}$	
3	Fund.	1.22	$G_{k,1}$	+ 1.35 $\Psi_0 Q_{k,2}$
4	Fund.	1.22	$G_{k,1}$	+ 1.35 $\Psi_0 Q_{k,3}$
5	Fund.	1.08	$G_{k,1}$	+ 1.35 $Q_{k,2}$
6	Fund.	1.08	$G_{k,1}$	+ 1.35 $Q_{k,3}$
7	Fund.	0.90	$G_{k,1}$	+ 1.35 $\Psi_0 Q_{k,2}$
8	Fund.	0.90	$G_{k,1}$	+ 1.35 $Q_{k,2}$
9	Fund.	0.90	$G_{k,1}$	+ 1.35 $Q_{k,3}$
10	Fund.	0.90	$G_{k,1}$	+ 1.35 $\Psi_0 Q_{k,3}$
11	Kar.	1.00	$G_{k,1}$	+ 1.00 $Q_{k,2}$
12	Kar.	1.00	$G_{k,1}$	+ 1.00 $Q_{k,3}$
13	Blij.	1.00	$G_{k,1}$	

GUNSTIGE WERKING PERMANENTE BELASTINGEN

BC Velden met gunstige werking

- 1 Geen
- 2 Alle velden de factor:0.90
- 3 Geen
- 4 Geen
- 5 Geen
- 6 Geen

GUNSTIGE WERKING PERMANENTE BELASTINGEN

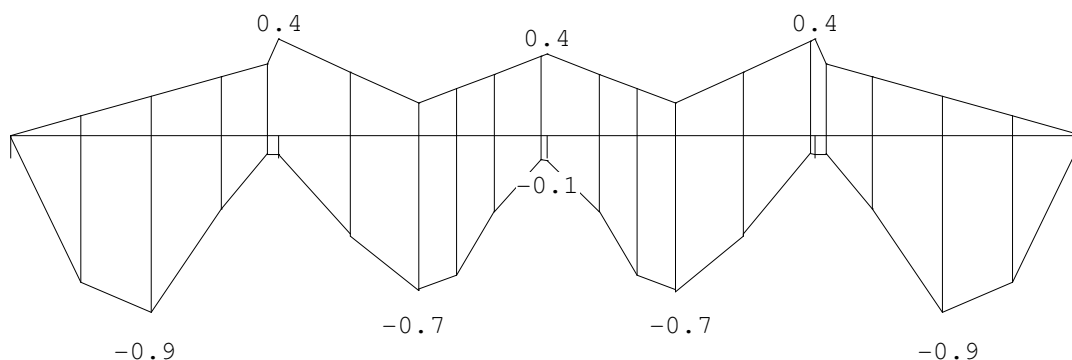
BC Velden met gunstige werking

- 7 Alle velden de factor:0.90
- 8 Alle velden de factor:0.90
- 9 Alle velden de factor:0.90
- 10 Alle velden de factor:0.90

OMHULLENDE VAN DE FUNDAMENTELE COMBINATIES

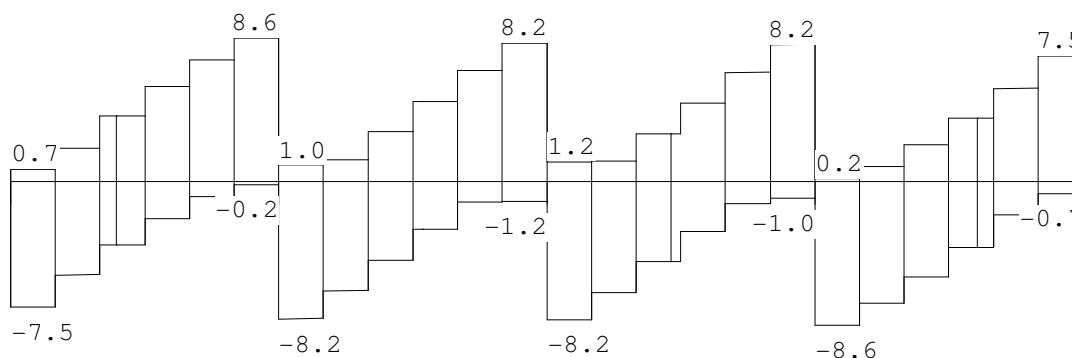
MOMENTEN

Ligger:1 Fundamentele combinatie



DWARSKRACHTEN

Ligger:1 Fundamentele combinatie



Fmin:-0.72 -1.14 -1.47 -1.14 -0.72
Fmax:9.5 9.5 9.5 9.5 9.5

REACTIES

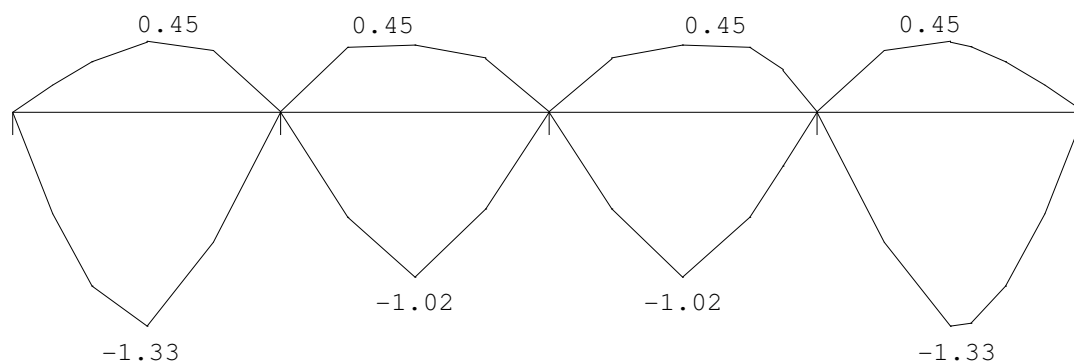
Ligger:1 Fundamentele combinatie

Stp	Fmin	Fmax	Mmin	Mmax
-----	------	------	------	------

1	-0.72	9.47	0.00	0.00
2	-1.14	9.52	0.00	0.00
3	-1.47	9.51	0.00	0.00
4	-1.14	9.52	0.00	0.00
5	-0.72	9.47	0.00	0.00

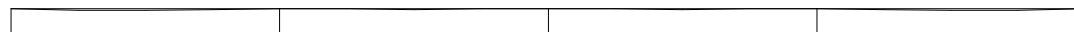
OMHULLENDE VAN DE KARAKTERISTIEKE COMBINATIES

VERPLAATSINGEN [mm] Ligger:1 Karakteristieke combinatie



OMHULLENDE VAN DE BLIJVENDE COMBINATIES

VERPLAATSINGEN [mm] Ligger:1 Blijvende combinatie



STAALPROFIELEN - ALGEMENE GEGEVENS

Ligger:1

Stabiliteit: Classificatie gehele constructie: Geschoord

MATERIAAL

Mat nr.	Profielnaam	Vloeisp. [N/mm ²]	Productie methode	Min. drsn. klasse
1	K60/40/2Z	235	Warmgewalst	1

Partiële veiligheidsfactoren:

Gamma M;0 : 1.00 Gamma M;1 : 1.00

KIPSTABILITEIT

Ligger:1

Staafl.	Plts. aangr.	l gaffel [m]	Kipsteunafstanden [m]
1	1.0*h	boven:	0.45 0.450
		onder:	0.45 0.450
2	1.0*h	boven:	0.45 0.450
		onder:	0.45 0.450
3	1.0*h	boven:	0.45 0.450
		onder:	0.45 0.450
4	1.0*h	boven:	0.45 0.450
		onder:	0.45 0.450

TOETSING SPANNINGEN

Ligger:1

Staaft nr.	Mat	BC	Sit	Kl	Plaats	Norm	Artikel	Formule	Hoogste toetsing U.C. [N/mm ²]	Opm.
1	1	6	4	1	Mz-max	EN3-1-1	6.2.8	(6.29)	0.628 148	< 150
2	1	6	10	1	Mz-max	EN3-1-1	6.2.8	(6.29)	0.544 128	
3	1	6	16	1	Mz-max	EN3-1-1	6.2.8	(6.29)	0.544 128	
4	1	6	22	1	Mz-max	EN3-1-1	6.2.8	(6.29)	0.628 148	